#### STATE OF NEW MEXICO BEFORE THE WATER QUALITY CONTROL COMMISSION

In the Matter of:

PROPOSED AMENDMENT TO 20.6.2 NMAC (Copper Rule) No. WQCC 12-01(R)

#### **EXHIBIT SCOTT – B-1**

# CLOSURE DISCUSSION TAILING DAMS AND MINE ROCK STOCKPILES

Technical Committee Presentation Copper Rules June 5, 2012





### **Jim Scott**

- Principal Geotechnical Engineer with URS for 35 years
- BS (Arizona State University) and MSCE (Purdue University)
- $\succ$  P.E. in NM, AZ, CO, and B.C.
- Mining industry experience includes engineering analyses and design for development, operation, reclamation, and closure projects
- URS Principal-In-Charge for work at the Bagdad Mine in AZ (Mammoth, Upper Mammoth, and Mulholland tailing impoundments)
- New Mexico experience includes work at Chino Mines Company, Cobre Mine, Tyrone Mine, Hidalgo Smelter, Questa Mine, and Four Corners Generating Facility





### **Closure Issues**

#### Tailing Dams

- NMOSE criteria
- Design storm events (surface water conveyance)
- Top surface grading
- Mine Rock Stockpiles
  - Non-water impounding
  - Investigations
  - Stability
  - Acceptable Factors of Safety





### New Mexico Office of the State Engineer Dam Safety Closure Criteria for Tailing Dams

- Jurisdictional tailing dams
- Closure or Reclamation Plan is prepared
- Plan addresses
  - Long-term stability (static and dynamic conditions)
  - Control of surface runoff to minimize erosion
  - Plan for long-term monitoring
  - Engineer to supervise construction of plan
- Construction completion
  - Completion report
  - Materials test data and photographs
  - As-built drawings
  - Certificate of completion



# Design Storm Events (Surface Water Conveyance)

- Spillways/diversion channels for TSF
  - 1/2 PMF PMF
- State-of-the-Practice for TSF and Rock Piles (surface channels/ditches)
  - NM Chino 100-yr return period
    - Tyrone 100-yr return period
    - Cobre 100-yr return period
    - Mining Act 100-yr return period (stream diversions)
  - AZ BADCT 100-yr return period
  - MSHA/OSM 100-yr return period
- Consider stability of channels/ditches
- Good maintenance program just as necessary as good design and construction



### Factors Influencing Closure Stability and Safe Performance of Tailing Dams

Shear Strength of Slope Materials/Foundation
Pore Pressure/Phreatic Surface Location
Slope Angle
Unit Weight of Materials in the Slope
Loading Condition (steady-state, seismic)
Factor of Safety (FS) (NMOSE)

 1.5 (steady-state)
 1.1 (seismic)

NMOSE criteria includes liquefaction potential evaluation



NOTES:

- STABILITY ANALYSES WERE PERFORMED WITH A COMPUTER USING THE SLOPE STABILITY PROGRAM "UTEXAS3" USING SPENCER'S METHOD.
- 2. PORE WATER PRESSURES IN TAILING DEFINED BY PIEZOMETRIC LINE ASSUMING HYDROSTATIC CONDITIONS.
- SEE FIGURE 3.1 FOR LOCATION OF STABILITY STUDY SECTION P2-P2'.

| ASSUMED MATERIAL PROPERTIES |  |                         |                                    |                   |
|-----------------------------|--|-------------------------|------------------------------------|-------------------|
| SOIL<br>TYPE<br>NO.         | MATERIAL DESCRIPTION                   | UNIT<br>WEIGHT<br>(pcf) | EFFECTIVE STRESS<br>SHEAR STRENGTH |                   |
|                             |  |                         | FRICTION<br>ANGLE<br>(degrees)     | COHESION<br>(psf) |
| 1                           | NATURAL FOUNDATION SOILS               | 135                     | 40                                 | 0                 |
| 2                           | TAILING SLIMES                         | 134                     | 35                                 | 0                 |
| 3                           | INTERLAYERED SANDS/SLIMES              | 132                     | 35                                 | 0                 |
| 4                           | UNDERFLOW SANDS                        | 120                     | 36                                 | 0                 |
| 5                           | STARTER DAM AND SLIMES/SEPARATION DIKE | 132                     | 35                                 | 0                 |

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### **TSF Top Surfaces**

#### Develop post-settlement contours

- Grading plan
- Drainage system
- Settlement due to
  - Drain down
  - Weight of top surface cover (2-3')

State-of-the-Practice top surface grades of 0.5 % for TSFs (largely driven by scale)





### **Mine Rock Stockpiles**

- Non-Water Impounding (different than tailing dams)
  - Does not impound process water
- Historical Background
  - First International Conference on Stability in Open Pit Mining, Vancouver, BC (1970)
  - Second International Conference on Stability in Open Pit Mining, Vancouver, BC (1971)
  - Third International Conference on Stability in Open Pit Mining, Vancouver, BC (1981)
  - Fourth International Conference on Stability in Open Pit Mining, Denver, CO (2001)
  - Canada Centre for Mineral and Energy Technology (CANMET) Pit Slope Manual – Waste Embankments (1977)
  - USDA Forest Service Guide Stability of Non-Water Impounding Mine Waste Embankments (1980)
  - SME, AIME Workshop Non-Impounding Waste Rock Dumps (1985)





### Mine Rock Stockpiles (cont'd.)

- British Columbia Mine Waste Rock Pile Research Committee (1991-1994)
  - Investigation and Design Manual (1991)
  - Dump Stability Rating Scheme
    - Dump Stability Class (I IV)
    - Failure Hazard (Negligible High)
  - Operating and Monitoring Manual (1991)
  - Methods of Monitoring (1992)
- Tailings and Mine Waste Conferences (yearly)
- First International Seminar on the Management of Rock Dumps, Stockpiles, and Heap Leach Pads, Perth, Australia (2008)



# Mine Rock Stockpile Investigations

- Investigations (field and laboratory)
  - Site characterization
  - Hydrology
  - Geology
  - Seismicity
  - Foundation Soils/Bedrock engineering properties
  - Mine Rock engineering properties
- Numerous investigation guides
  - BC Guidelines Mined Rock and Overburden Piles Investigation and Design Manual (1991)
  - SME Design of Non-Impounding Mine Waste Dumps (1985)
  - AZ BADCT Guidance Manual (2004)
  - MSHA Engineering and Design Manual, Coal Refuse Disposal Facilities (2009)
  - California DMG Special Publication 117 (2002)
  - TRB Landslide Analysis and Control, Special Report 176 (1978)





### Mine Rock Stockpile Stability

- Factors Affecting Stability
  - Configuration (height, volume, slope angle)
  - Foundation Slope/Confinement
  - Foundation Conditions
  - Mine Rock Properties (durable igneous/metamorphic rocks, low susceptibility to weathering, low fines, free draining, high strength)
  - Construction Method
  - Piezometric/Climatic Conditions
  - Seismicity
- Material Strengths
  - Empirical Correlations (Leps (1970), Barton and Kjaernsli (1981), Barton (2008), Hoek (1990), Hoek and Brown (1997))
  - Large Scale Direct Shear Tests
  - Triaxial Shear Tests
  - In situ (BPTs, PMTs, NALPTs, SPTs)
- Phreatic/Piezometric Conditions
  - Test Holes
  - Monitoring Wells
  - Piezometers
  - Observations



# Mine Rock Stockpiles Acceptable Factors of Safety

- NM Mining Act (1999)
  - Piles shall be constructed and maintained to minimize mass movement
- > NMDOT
  - Use AASHTO design criteria
  - 1.3 for static loads
  - 1.1 for seismic loads
- AZ BADCT Guidance Manual (2004)
  - Establish whether or not discharge can occur
  - Static stability
    - 1.5 w/o testing
    - 1.3 w/testing (material shear strengths)
  - Dynamic stability
    - ≥1.1 w/o testing
    - □ ≥ 1.0 w/testing (material shear strengths)



# Mine Rock Stockpiles Acceptable Factors of Safety (cont'd).

- British Columbia Mine Waste Rock Pile Research Committee
  - Consider the following factors
    - Shear strength
    - Material composition
    - Foundation conditions and geometry
    - Short-term (during construction) vs. long-term (reclamation) slopes
    - Consequence of failure
    - Field control
    - Engineering judgment
  - Factor of Safety
    - Long-term static 1.3 1.5
    - Pseudo-static (earthquake) 1.0 1.3
    - Ranges reflect understanding of site-specific conditions





## State-of the-Practice Factors of Safety Mine Rock Stockpiles

Western U.S. hard rock copper mines
FS=1.3 (static) and 1.0 (seismic)
Site-specific investigations (field and lab)

- Mine rock properties
- Foundation conditions









# **QUESTIONS / COMMENTS?**



